

Metoda elementów skończonych (MES1)

Wykład 8A. 3D element kratownicy

04.2022

Element skończony pręta kratownicy 3D



2

Przemieszczenia węzła 1

$$U_{1} = q_{1} \cos \alpha$$

$$V_{1} = q_{1} \cos \beta$$

$$W_{1} = q_{1} \cos \beta$$

$$W_{1} = q_{1} \cos^{2} \alpha$$

$$U_{1} \cos \alpha = q_{1} \cos^{2} \alpha$$

$$V_{1} \cos \beta = q_{1} \cos^{2} \beta$$

$$W_{1} \cos \beta = q_{1} \cos^{2} \beta$$

$$W_{1} \cos \beta = q_{1} \cos^{2} \beta$$

$$W_{2} \cos \alpha + V_{1} \cos \beta + W_{2} \cos \gamma = q_{1}$$

Przemieszczenia węzła 1 i 2

$$q_{1} = a \cdot u_{1} + b \cdot v_{1} + c \cdot w_{1} + 0 \cdot u_{2} + 0 \cdot v_{2} + 0 \cdot w_{2}$$

$$q_{12} = 0 \cdot u_{1} + 0 \cdot v_{1} + 0 \cdot w_{1} + a \cdot u_{2} + b \cdot v_{2} + c \cdot w_{2}$$

Z

91

 W_1

1

u,

Wektor przemieszczeń węzłowych elementu kratownicy

 $q_{11} = a \cdot u_1 + b \cdot v_1 + c \cdot w_1 + 0 \cdot u_2 + 0 \cdot v_2 + 0 \cdot w_2$ $q_{12} = 0 \cdot u_1 + 0 \cdot v_1 + 0 \cdot w_1 + \alpha \cdot u_2 + b \cdot v_2 + c \cdot w_2$ $\begin{cases} q_1 \\ q_2 \\ e \end{cases} = \begin{bmatrix} a & b & c & 0 & 0 & 0 \\ 0 & 0 & a & b & c \end{bmatrix} \begin{cases} u_1 \\ v_1 \\ w_1 \\ u_2 \\ v_2 \\ v_2 \end{cases}$ $\{q_{i}\}_{e} = [T_{t}]_{e} \cdot \{q_{i}\}_{e}$ $\begin{array}{c} L_{q} J_{e} = L_{q} J_{9} J \cdot \begin{bmatrix} T_{t} \end{bmatrix}_{e}^{T} ; \\ 1 \times 2 & 1 \times 6 & 6 \times 2 \end{bmatrix} \cdot \begin{bmatrix} T_{t} \end{bmatrix}_{e}^{T} ; \\ \begin{bmatrix} T_{t} \end{bmatrix}_{e}^{T} = \begin{bmatrix} v & U \\ b & 0 \\ c & 0 \\ 0 & a \\ 0 & b \\ c \end{bmatrix}$ Macierz transformacji

Energia sprężysta elementu kratownicy:

$$\begin{aligned} & \iint_{e} = \frac{4}{2} \begin{bmatrix} q_{1} \end{bmatrix}_{e} \begin{bmatrix} k \end{bmatrix}_{e} \begin{bmatrix} q_{2} \end{bmatrix}_{e} = \frac{4}{2} \begin{bmatrix} q_{3} \end{bmatrix}_{e} \begin{bmatrix} T_{1} \end{bmatrix}_{e} \begin{bmatrix} r_{1} \end{bmatrix}_{e} \begin{bmatrix} q_{3} \end{bmatrix}_{e} = \frac{4}{1 \times 2} \begin{bmatrix} q_{3} \end{bmatrix}_{e} = \frac{4}{2 \times 2} \begin{bmatrix} q_{3} \end{bmatrix}_{e} \begin{bmatrix} q_{3} \end{bmatrix}_{e} = \frac{4}{1 \times 6} \begin{bmatrix} q_{3} \end{bmatrix}_{e} \begin{bmatrix} r_{1} \end{bmatrix}_{e} \begin{bmatrix} q_{2} \end{bmatrix}_{e} \begin{bmatrix} r_{1} \end{bmatrix}_{e} \begin{bmatrix} r_{2} \\ r_{2} \end{bmatrix}_{e} \begin{bmatrix} r_{2} \end{bmatrix}_{e} \begin{bmatrix} r_{2} \end{bmatrix}_{e} \begin{bmatrix} r_{2} \end{bmatrix}_{e} \begin{bmatrix} r_{2} \\ r_{2} \end{bmatrix}_{e} \begin{bmatrix} r_{2} \end{bmatrix}_{e} \begin{bmatrix} r_{2} \\ r_{2} \end{bmatrix}_{e} \end{bmatrix}_{e} \begin{bmatrix} r_{2} \\ r_{2} \end{bmatrix}_{e} \end{bmatrix}_{e} \begin{bmatrix} r_{2} \\ r_{2} \end{bmatrix}_{e} \begin{bmatrix} r_{2} \\ r_{2} \end{bmatrix}_{e} \end{bmatrix}_{e} \end{bmatrix}_{e$$

Przykład Zbuduj model MES przestrzennej kratownicy. Znajdź przemieszczenia węzłowe, naprężenia, siły wewnętrzne i reakcje



Model MES

ELENENT	NODES
1	$ (0,0,0) \rightarrow (0,0,400) $
2	$2(-400,0,0) \rightarrow 4(0,0,400)$
3	3(0,-300,0)->(4)(0,0,400)



Macierz sztywności elementu 1

ELEMENT [] , L1 = 400 mm $a_1 = \frac{0-0}{L_1} = 0$; $b_1 = \frac{0-0}{L_1} = 0$; $C_1 = \frac{400-0}{400} = 1$ $\begin{bmatrix} T_{t} \\ t_{t} \end{bmatrix}_{1} = \begin{bmatrix} 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 \end{bmatrix}$

Macierz sztywności elementu 2 ELEHENT 2 $l_2 = \sqrt{(0 - (-400))^2 + (0 - 0)^2 + (400 - 0)^2} = 40072 \text{ mm}$ $a_2 = \frac{0 - (-400)}{400/2} = \frac{12}{2}, \quad b_2 = \frac{0 - 0}{400/2} = 0, \quad c_2 = \frac{400 - 0}{400/2} = \frac{12}{2}$ $\begin{bmatrix} T_{t} \\ 2 \\ 2 \\ 2 \\ 2 \\ x_{6} \end{bmatrix} = \begin{bmatrix} \frac{7}{2} & 0 & \frac{7}{2} & 0 & 0 & 0 \\ 0 & 0 & \frac{7}{2} & 0 & \frac{7}{2} \\ 0 & 0 & 0 & \frac{7}{2} & 0 & \frac{7}{2} \end{bmatrix}$ $= \frac{\left[\frac{1}{2} \\ 0 \\ -\frac{1}{2} \\ 0 \\ -\frac{1}{2} \\ 0 \\ -\frac{1}{2} \\ -\frac{1}{2} \\ 0$ LKgj kg J2 9

Macierz sztywności elementu 3 ELEMENT $l_{3} = \sqrt{(0-0)^{2} + (0-(-300))^{2} + (400-0)^{2}} = 500 \text{ mm}$ $u_{3} = \frac{0-0}{500} = 0, \quad b_{3} = \frac{0-(-300)}{500} = \frac{3}{5} = 0.6, \quad c_{3} = \frac{400-0}{500} = 0.8$ 3 $\begin{bmatrix} T_{\pm} \end{bmatrix}_{3} = \begin{bmatrix} 0 & 0.6 & 0.8 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0.6 & 0.8 \end{bmatrix}$ $\begin{bmatrix} k_{9} \end{bmatrix}_{3} = \frac{EA}{l_{3}} \begin{bmatrix} 0 & 0 & 0 & 0 & 0 \\ 0 & 0.36 & 0.48 & 0 & -0.36 & -0.48 \\ 0 & 0.48 & 0.64 & 0 & -0.48 & -0.64 \\ 0 & 0 & 0 & 0 & 0 \end{bmatrix}$ $[k_{g}]_{3}^{*} = [0]_{6*6}$ 0 -0.36 -0.48 0 0.36 0.48 -0.48-0.64 () 0.48 0.64 10

Globalna macierz sztywności modelu kratownicy



 $[K] = [k_9]_1^* + [k_9]_2^* + [k_9]_3^* = 12 \times 12$



Układ równań:

 $\begin{bmatrix} K \end{bmatrix} = EA \begin{bmatrix} \frac{1}{2l_2} & 0 & \frac{1}{2l_2} \\ 0 & \frac{0.36}{l_3} & \frac{0.48}{l_3} \\ \frac{1}{2l_2} & \frac{0.48}{l_3} & \frac{1}{l_4} + \frac{1}{2l_2} + \frac{0.64}{l_3} \end{bmatrix}$ $\begin{bmatrix} K \end{bmatrix} \cdot \begin{bmatrix} U_4 \\ V_4 \\ = \end{bmatrix} \begin{bmatrix} 2F \\ F \\ 0 \end{bmatrix}$ $\begin{cases} \frac{1}{2l_2} \cdot u_4 + 0 \cdot v_4 + \frac{1}{2l_2} u_4 = \frac{2F}{EA} & |\cdot 2l_2 \\ 0 \cdot u_4 + \frac{0.36}{l_3} v_4 + \frac{0.48}{l_3} u_4 = \frac{F}{EA} & |\cdot \frac{100}{36} u_3 \\ \frac{1}{2l_2} \cdot u_4 + \frac{0.48}{l_3} u_4 + \left(\frac{1}{l_4} + \frac{1}{2l_2} + \frac{0.64}{l_3}\right) u_4 = 0 \end{cases}$

 $\begin{aligned} \mathcal{U}_{4} + \mathcal{W}_{4} &= \frac{4Fl_{2}}{EA} &= \mathcal{U}_{4} = \frac{4Fl_{2}}{EA} - \mathcal{W}_{4} \\ \mathcal{V}_{4} + \frac{4}{3}\mathcal{W}_{4} &= \frac{100Fl_{3}}{36EA} &= \mathcal{V}_{4} = \frac{100Fl_{3}}{36EA} - \frac{4}{3}\mathcal{W}_{4} \end{aligned}$ $\int \frac{1}{2b} \frac{4}{24} + \frac{0.48}{1_3} v_q + \left(\frac{1}{4} + \frac{1}{2b} + \frac{0.69}{1_3}\right) w_q = 0$ $\frac{1}{26} \cdot \left(\frac{4Fl_2}{EA} - W_4\right) + \frac{0.98}{L_2} \cdot \left(\frac{100}{36} \frac{Fl_3}{EA} - \frac{4}{3}W_4\right) + \left(\frac{1}{4} + \frac{1}{2L_2} + \frac{0.67}{L_3}\right)W_q = 0$ $\frac{2F}{EA} + \frac{4F}{3EA} + \left(\frac{1}{l_1} + \frac{4}{2l_2} + \frac{0.69}{l_3} - \frac{1}{2l_2} - \frac{0.69}{l_3}\right) w_4 = 0$ $W_{4} = -\left(\frac{2F}{EA} + \frac{4F}{3EA}\right)_{4} = -\frac{10F_{4}}{3EA} = -0.1 \text{ mm}$ $U_{4} = \frac{4Fl_{2}}{EA} + \frac{10Fl_{1}}{3EA} = \frac{12Fl_{2}}{3EA} + \frac{10Fl_{1}}{3EA} = \frac{(12l_{2} + 10l_{1})F}{3EA} = 0.27mm$ $V_4 = \frac{100 \text{ FL}_3}{36 \text{ EA}} - \frac{4}{3} \cdot \left(-\frac{10 \text{ FL}}{3 \text{ EA}}\right) = \frac{100 \text{ FL}_3}{36 \text{ EA}} + \frac{40 \text{ FL}_4}{9 \text{ EA}} = \frac{(100 \text{ L}_3 + 160 \text{ L}_4)\text{ F}}{36 \text{ EA}} = 0.2375 \text{ mm}$

0 0 Reakcje: $[K] \cdot [q] = \{F_{2}^{2}\}$ 0 0 0 0 0 $R_1 = 0$, $R_2 = 0$ 0 0 0 0 0 0 0 0 $R_3 = -\frac{EA}{I_1} \cdot N_4 = -\frac{EA}{I_1} \cdot (-\frac{I_2}{2} \cdot \frac{FL_4}{FA}) = \frac{I_2}{3} \cdot F = 5000 \text{ N}$ 0 0 0 0 0 $R_{4} = -\frac{EA}{2L_{0}} \cdot U_{4} - \frac{EA}{2L_{0}} U_{4} = -\frac{EAF}{2L_{0}} \left(\frac{12L_{0} + 10L_{1}}{3EA} - \frac{10L_{1}}{3EA} \right) = -2F = -3000N$ $R_5 = 0$, $R_6 = -\frac{EA}{2L_0}u_4 - \frac{EA}{2L_0}w_4 = R_4 = -2F = -3000N$ $R_7 = 0$, $R_8 = -\frac{0.36EA}{l_2}$, $V_4 - \frac{0.48EA}{l_2}W_4 =$ $= -0.36 EA \cdot 100 (l_3 + 1.6 l_4)F + 0.48 EA \cdot 10 F l_4 = -F = -1500N$ $R_g = -0.48 EA \cdot V_4 - 0.64 EA W_4 = U_2$ $= - \underbrace{0.48EA}_{L_3} \cdot \underbrace{100(l_3 + 1.6l_4)F}_{36EA} + \underbrace{0.64EA}_{L_2} \cdot \underbrace{10Eh}_{3EA} = -\underbrace{4F}_{3} = -2000N$



Równowaga sił:

$$2F_{x}=0$$
: $-2F+2F=0$
 $2F_{y}=0$: $-F+F=0$
 $2F_{y}=0$: $-F+2F-4F=0$
 $2F_{y}=0$: $-2F-4F=0$
 $2F_{y}=0$
 $2F_{y}=0$

Rozwiązanie w elemencie 1:

$$\begin{bmatrix} \mathbf{A} \\ \{q\}_{1}^{2} = \begin{bmatrix} \mathbf{T}_{t} \end{bmatrix}_{1}^{2} \cdot \begin{cases} u_{1} \\ w_{1} \\ w_{2} \\ w_{1} \\ w_{2} \\ w_{1} \\ w_{2} \\ w_{2} \\ w_{2} \\ w_{2} \\ w_{2} \\ w_{3} \\ w_{4} \\ w_{$$

 $\vec{b}_1 = \vec{E} \cdot \vec{E}_1 = -\frac{10}{3} \vec{F}_n = -50 \, \text{MR} \qquad N_1 = \vec{b}_1 \cdot \vec{A} = -\frac{10}{3} \vec{F}_n = -5000 \, \text{N}$ Ściskanie (możliwe wyboczenie?) Siła krytyczna w elemencie 1:

$$\mathbf{P_{kr}} = \frac{\pi^2 E J}{L_1^2}$$



$$P_{kr} = \frac{\pi^{2} E \cdot 10^{4} mm^{4}}{12 \cdot 400^{2} mm^{2}} = \frac{\pi^{2} \cdot 2 \cdot 10^{5} \cdot 10^{4} Nmm^{2}}{12 \cdot 16 \cdot 10^{4} mm^{2}} = 10281 N$$

Współczynnik
bezpieczeństwa $n = 2$
 $|N_{1}| < \frac{P_{kr}}{N_{1}}$

Rozwiązanie w elemencie 2:

$$\begin{cases} q_{1}^{2} = \begin{bmatrix} \Gamma_{e} \end{bmatrix}_{2} & \begin{cases} u_{2} \\ v_{2} \\ w_{2} \\ u_{4} \\ v_{4} \\ w_{4} \end{cases} = \begin{bmatrix} \frac{1}{2} & \frac{1}{2} &$$

$$\mathcal{E}_{2} = \frac{12}{2l_{2}} \left(\frac{u_{4} + w_{4}}{4} \right) = \frac{12}{2l_{2}} \cdot \left(\frac{(12l_{2} + 10l_{4})F}{3EA} - \frac{10Fl_{4}}{3EA} \right) = \frac{212}{EA} = 0.212 \cdot 10^{-3}$$

 $G_2 = EE_2 = 42.43 MPa_1$

 $N_2 = G_2 - A = 4243N$

Rozwiązanie w elemencie 3:

$$\mathcal{E}_{3} = \frac{1}{L_{3}} \left(0.6 \cdot \frac{(100 L_{3} + 160 L_{4})F}{36 EA} - 0.8 \cdot \frac{10}{3} \frac{FL_{4}}{EA} \right) = \frac{5F}{3EA} = 0.125 \cdot 10^{-3}$$

 $G_3 = E \cdot \xi_3 = 25 M Pa$

 $N_3 = 6_3 \cdot A = 2500N$

Porównanie wyników obliczeń z tymi uzyskanymi w programie ANSYS



Modyfikacja modelu w programie ANSYS

(AVG)

-50

-39.7304

-29.4608 -19.1912

-8.9216

11.6176

21.8872

SOLUTION

(AVG)

1.348



- 50 MA	32.1568 42.4264
3000	PLOT NO. 1 NODAL SOLUTIO STEP=1 SUB =1 TIME=1 SX (AVG) RSYS=0 PowerGraphics EFACET=1 AVRES=Mat DMX =.385255 SMN =-30 SMX =42.4264 U F -30
	-21.9526 -13.9052 -5.85786 2.18951 10.2369 18.2843 26.3316 34.379 42.4264



Rozbudowa modelu w programie ANSYS



Rozbudowa modelu w programie ANSYS







Build 19.2 APR 19 2023 16:28:02 PLOT NO. 1 ELEMENT SOLUTION STEP=1 SUB =1 TIME=1 (NOAVG) SX RSYS=0 PowerGraphics EFACET=1 DMX = 6.77589 SMN = -180SMX =90 F -180-150 -120 -90 -60 -30 0 30 60 90

Rozciąganie rozbudowanego modelu w programie ANSYS









Skręcanie rozbudowanego modelu w programie ANSYS







витта та.ч APR 19 2023 18:00:56 PLOT NO. 1 ELEMENT SOLUTION STEP=1 SUB =1 TIME=1 SX (NOAVG) RSYS=0 PowerGraphics EFACET=1 DMX = 2.58834SMN = -32.0156SMX =25 U F -32.0156 -25.6806-19.3455-13.0104 -6.67535 -.340276 5.99479 12.3299 18.6649 25